

Highlights

- In civil engineering structural safety often relies on safety coefficients.
- Damage tolerance may help to improve structural safety.
- Damage tolerance is presented for designing prestressing anchorages (PA).
- 2D and 3D FE models are used for assessing damage tolerance of PA.
- A specific anchorage geometry is analysed to show the effectiveness of this approach.

Damage tolerance in engineering components: Implementation to anchor heads

M.A. Olivares¹, J.C. Gálvez¹, F.Suárez²

¹*Universidad Politécnica de Madrid, Departamento de Ingeniería Civil-Construcción, E.T.S.I. de Caminos, Canales y Puertos, C/ Prof. Aranguren s/n, 28040 Madrid, Spain*

²*Departamento de Ingeniería Mecánica y Minera. Universidad de Jaén. Campus Científico-Tecnológico de Linares. Cinturón Sur 23700 – Linares (Jaén), Spain*

Abstract

Damage tolerance is commonly used as a design criterion in some engineering fields, such as aerospace engineering, but is not usual in civil engineering, where structural safety often relies on safety coefficients. This work presents a study on how using damage tolerance for designing post-tensioning anchor heads, usual mechanisms in many civil engineering works, can be useful for building safer structures. Damage is simulated using 2D and 3D finite element models, considering axisymmetric damage and single damage. The influence of several aspects, such as the crack depth, the crack orientation angle or the shape of the anchorage, is studied. Finally, a specific anchorage geometry is analysed using the damage tolerance concept to show that this approach may help in the design of this type of elements.

Keywords: Damage tolerance, post-tensioning anchor head, stress intensity factor, Finite Element Method

1. Introduction

The concept of damage tolerance in engineering design is based on the assumption that any structural element can have flaws that can limit their life with usage [1]. Design based on damage tolerance is a common practice in aerospace engineering [2, 3] and nuclear engineering [4, 5], but not so usual in more traditional fields, such as civil engineering, where design is based on considering components without any defect.

Nevertheless, in real practice, structural components do present defects due to the manufacturing process, transport or assembly, for instance. These possible imperfections are usually considered to be covered by safety coefficients used in the design project, which lead to designing elements under magnified loads and considering materials with lower strength properties. This approach provides a simple procedure for computing the strength of structural components but provides no guidance to estimate its life when a crack grows as a consequence of an overload, repeated loads or under an aggressive environment.

As observed by Valiente and Elices [6], the usual design criteria, such as yield strength, ultimate strength and ductility, may not avoid problems leading to brittle fracture. This fact has

37 attracted the attention of several authors in the last years and interesting contributions focused
38 on the damage tolerance concept applied in the construction field can be found analysing
39 structural details, such as post-tensioning systems [7], bolted joints [8], prestressing steel wires
40 [9], rods [10], composite elements under impact loading [11] or railway axles [12-16], proving
41 that this approach can help with the maintenance program of railway infrastructures.

42 More specifically, the concept of damage tolerance can be particularly useful to study
43 anchorage systems used in prestressed concrete elements [17] or prestressed systems anchored
44 to rock [18], since these elements, which work under high stress states, are prone to early
45 damage if small defects are present.

46 In this paper, a simple methodology is described for designing structural elements based on the
47 concept of damage tolerance. A post-tensioning anchorage used in prestressed concrete
48 elements is analysed as a case study to illustrate this methodology. Damage, such as corrosion
49 pits or dents, is idealised as tiny cracks, a hypothesis from the safe side. When brittle fracture
50 occurs – the worst scenario – fracture loads can be computed using linear elastic fracture
51 mechanics. This work provides some clues for: (a) computation of stress intensity factors in
52 involved geometries, such as anchor heads and (b) the measurement of the material fracture
53 toughness under the corresponding working conditions. The paper ends with an example where
54 the damage tolerance of an anchor is evaluated based on values taken from actual failures.

55 **2. Damage simulation**

56 **2.1. The damaged component**

57 Steel tendons with end anchorages are used to transfer compressive loads to concrete. The
58 performance of anchorages is a complex problem and, in general, is designed by experience,
59 tests, and usage, rather than by theory [19].

60 As already mentioned, defects—in the form of corrosion pits, scratches or dents— can appear
61 and act as stress raisers, similar to small cracks. Such cracks, depending on the stress acting on
62 them or on the environment, can grow and become unstable, leading to the eventual anchorage
63 failure.

64 To apply the concept of damage tolerance, the anchor head has to be modelled with an initial
65 damage and a crack is probably the simplest approach. The safest procedure is to place the
66 crack in the worst place, usually inside the most tensile stressed region of the anchor head.
67 With such approach, two anchor head geometries are evaluated in this research; a circular plate
68 and a simple anchor head.

69 **2.2. A circular plate transversally loaded**

70 A transversally loaded circular plate is an idealized structural component that can be found in
71 practice. The purpose of this section is to evaluate when an initial defect in the form of a crack
72 will start growing. This process is a function of the plate geometry, the crack size and its
73 location.

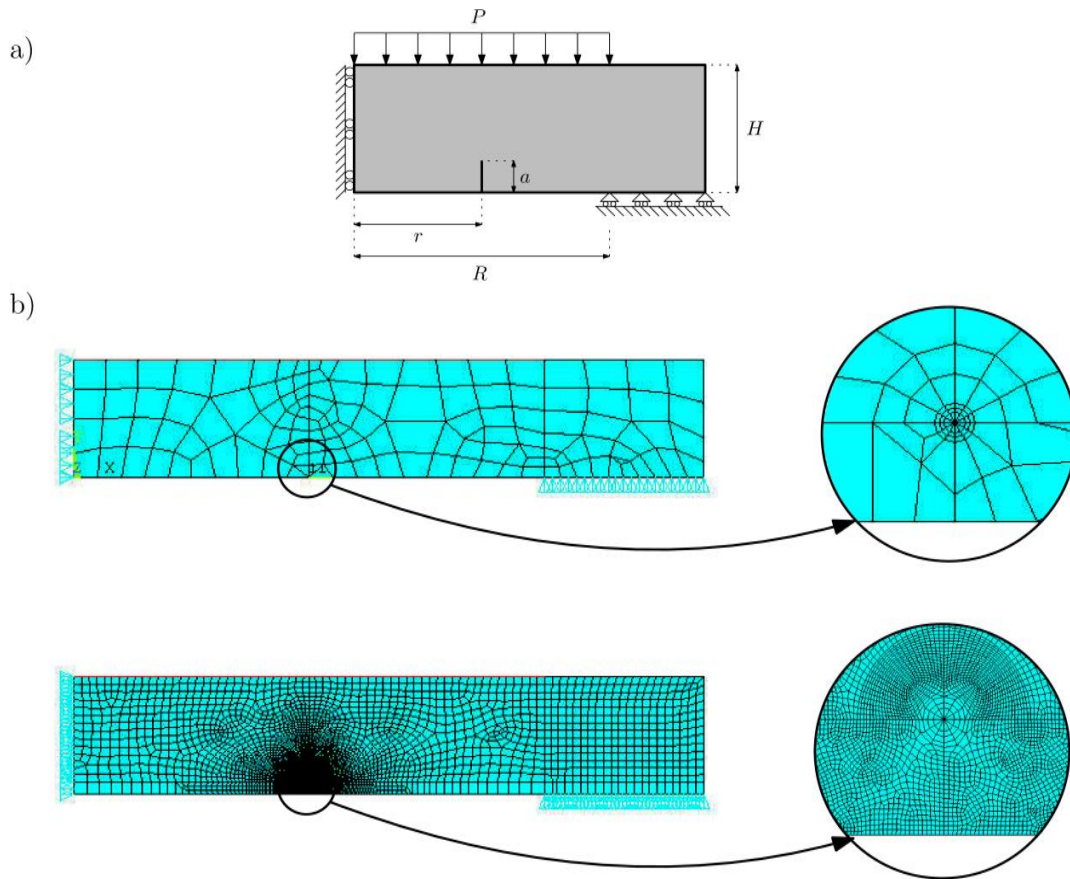
74 Two types of cracks were taken into consideration; a simple crack and a circular crack
75 surrounding the whole anchor head. Both of them are located on the tensile face. Consequently,
76 two types of finite element models were used: a two dimensional axisymmetric model for the
77 circular crack and a three dimensional model for the simple crack. The circular crack is
78 considered here because of its simplicity and is used as a simple procedure to find upper or
79 lower bounds of the problem.

80 2.2.1. Two-dimensional axisymmetric calculation

81 A disc of radius $1.34R$ and thickness H is considered and five values of H/R
82 = 0.25, 0.50, 0.75, 1.00 and 1.25. A uniform load P is distributed on a circle of radius R . The
83 disc is simply supported and nine different crack sizes are analysed; $a/H = 0.001, 0.002, 0.005,$
84 $0.01, 0.02, 0.05, 0.1, 0.2$ and 0.5 , where a stands for the crack depth. Cracks are located on the
85 tensile side of the plate, at distance r from the symmetric axis of the plate. Two different
86 locations were considered, $r = R/2$ and $R/4$ (see Figure 1a).

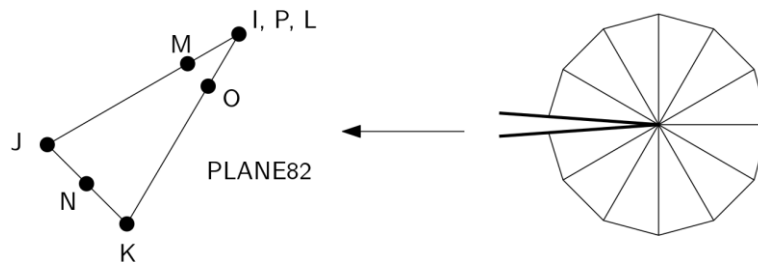
87 The model was built with the finite element code ANSYS. Quadrilateral elements of eight
88 nodes were used in order to generate the finite element mesh. The mesh at the crack location
89 is refined due to the stress singularity at the crack tip.

90 As shown in Figure 2, the mesh at the crack tip was generated by means of 12 quarter point 8-
91 node quadrilateral elements forming a rosette on which every element shares one of its nodes
92 at the crack tip location. This allows evaluating the stress intensity factor by means of the fit
93 of the nodal displacements in the vicinity of the crack tip. The size of the first row of elements
94 was set to $1/10$ of the crack depth a in order to obtain accurate values of the stress intensity
95 factors (SIF).



96 **Figure 1:** a) Dimensions, load and boundary conditions of the model; b) mesh refinement at
 97 the crack tip.

98



99

100 **Figure 2:** 2D mesh at the crack tip.

101 The remaining plate geometry was meshed by means of conventional quadrilateral elements of
 102 eight nodes. The element size was increased uniformly from the crack tip to the surface of the
 103 plate (Figure 1). An iterative mesh refinement process was carried out in order to reduce the
 104 mesh discretization error. The target was to meet a stress intensity factor (SIF) increment
 105 between two consecutive finer meshes less than 5%. Figure 1 shows two finite element models
 106 corresponding to different steps in the mesh refinement process.

107 The stress field in a plate subjected to bending strongly depends on the material through the
 108 Poisson's ratio ν and, consequently, the SIF values in cracked anchorages are expected to
 109 depend on this value.

110 The plate material in these models was defined as linear elastic with a Young's modulus E of
 111 200 GPa and a Poisson's ratio ν of 0.4, which is the Poisson's ratio of polymethyl-methacrylate
 112 (PMMA), that was used to check the extent and accuracy of the numerical approach.

113 For each analysed case, the equivalent SIF at the crack tip was obtained by means of the
 114 combination of K_I and K_{II} according to expression (1), which defines a crack propagation
 115 criterion when two different modes are present [20]. Values of K_I and K_{II} are obtained by means
 116 of the displacement extrapolation technique [21].

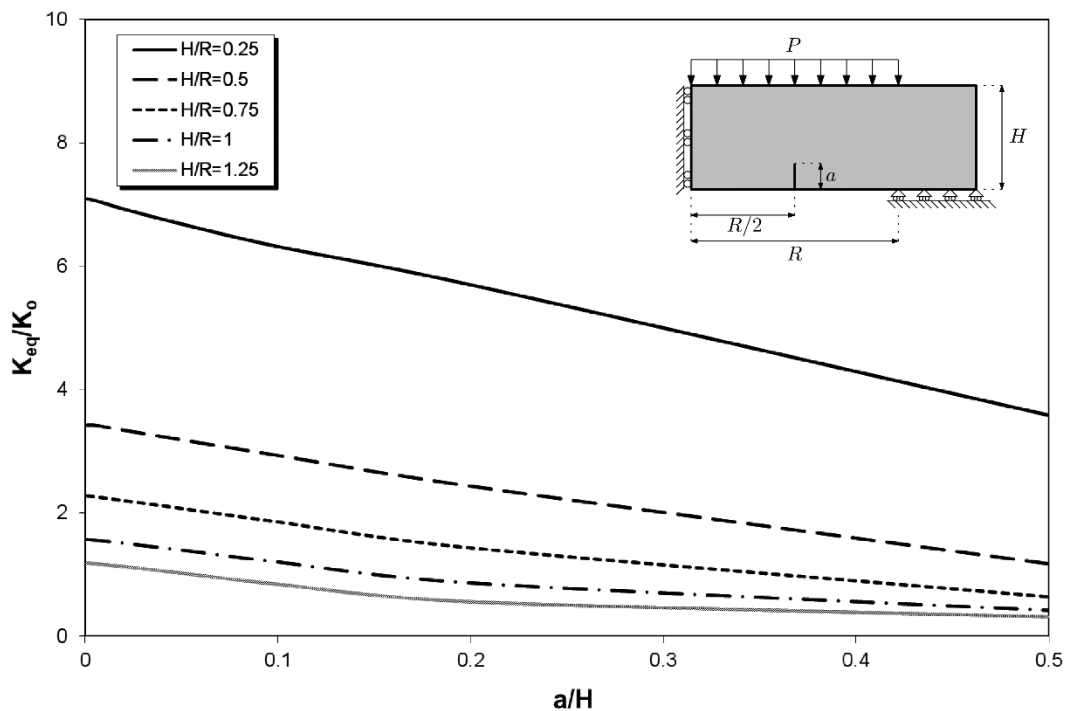
$$117 \quad K_{eq} = (K_I^2 + K_{II}^2)^{1/2} \quad (1)$$

118 The equivalent SIF is then normalized by the factor K_0 to get a dimensionless value. The value
 119 of K_0 is obtained by means of expression (2):

$$120 \quad K_0 = P (\pi a)^{1/2} \quad (2)$$

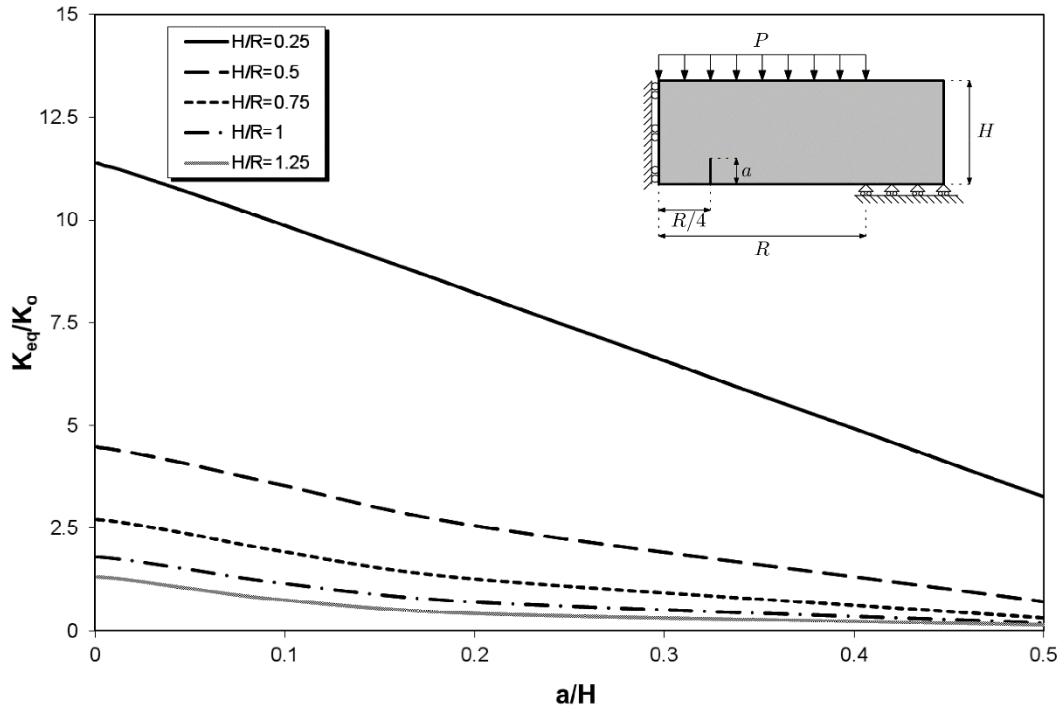
121 where P is the applied load on the upper face of the plate.

122 Figures 3 and 4 permit identifying the effect of the plate thickness H on the SIF. Figure 3
 123 presents the normalized equivalent SIF against the dimensionless depth of the crack a/H for
 124 those cracks located at distance $0.5R$ from the symmetric axis. Figure 4 shows analogous results
 125 for those cracks located at distance $0.25R$ from the symmetric axis.



126

127 Figure 3: Effect of the plate thickness on the SIF (Cracks placed at $R/2$).



128

129

Figure 4: Effect of the plate thickness on the SIF (Cracks placed at $R/4$).

130

131 It can be observed that the smaller the plate thickness H , the smaller the damage tolerance for
 132 an equal relative depth of the crack a/H . When the plate is thicker, part of the load can be
 133 directly transferred to the external ring support, which reduces bending in the intermediate
 134 region between the symmetry axis and the external support. Since the bending of the plate is
 135 what induces tensile stresses at the lower part of the plate, thus opening and propagating
 136 cracking, reducing this mechanism leads to a smaller value of SIF, which increases damage
 137 tolerance of the plate.

138 2.2.2. Three-dimensional axisymmetric calculation

139 A three dimensional calculation with a simple inserted crack was carried out, as shown in
 140 Figure 5. The crack had a semi-elliptical shape, with a major semi axis a and a minor semi axis
 141 b . To be consistent with the experimental work, the depth of the crack and the ratio between
 142 the major and the minor axis were set to 5.7 mm and 2.6 respectively, which are the average
 143 values experimentally measured in tested specimens.

144 A disc of radius $1.34R$ and thickness H is considered, with R being the radius of the circular
 145 area over which the distributed load P is applied (see details at Figure 1). Three values of H/R
 146 ratio are analysed; $H/R = 0.25, 0.50$ and 0.75 . Cracks are located on the tensile side of the plate,
 147 at a distance r from the symmetric axis of the plate. Three different locations are considered $r =$
 148 $R/2, R/4$ and $0.88R$.

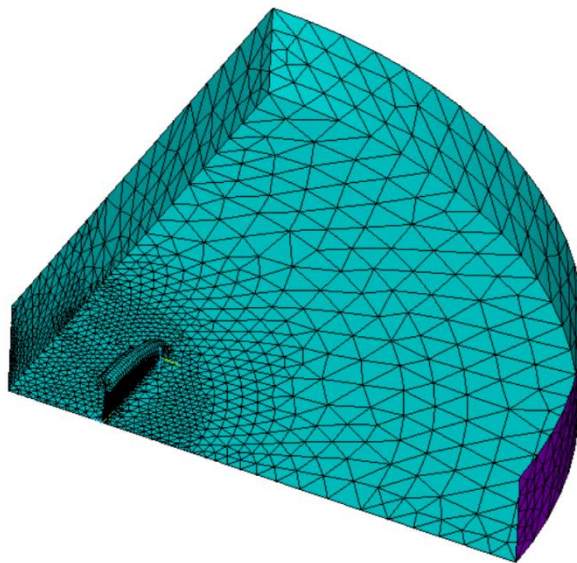
149 The calculation was made with the finite element code ANSYS. Tetrahedral elements of twenty
 150 nodes were used in order to generate the finite element mesh.

151 The mesh at the crack location requires special discretisation due to the stress singularity at the
152 crack tip. Figure 5a shows how the mesh is refined around the crack tip and Figure 5b shows
153 the elements used at the crack tip, which correspond to hexahedral concentric layers with
154 collapsed quarter-point elements at the innermost layer. The crack-front domains were filled
155 with a concentric 20-nodes hexahedral mesh of three layers. Standard elements are used except
156 for the collapsed singular quarter-point elements in the innermost layer connected to the crack-
157 front. This allows obtaining the stress intensity factor by means of the fit of the nodal
158 displacements in the vicinity of the crack tip. The size of the first row of elements was set to
159 1/10 of the crack depth a in order to give an accurate value of the stress intensity factors (SIF).
160 The number of elements along the crack front was set to keep the aspect ratio of the elements
161 within acceptable values and avoid numerical problems.

162 The remaining plate geometry was meshed by means of conventional quadrilateral elements of
163 eight nodes. The element size was uniformly increased from the crack tip to the surface of the
164 plate.

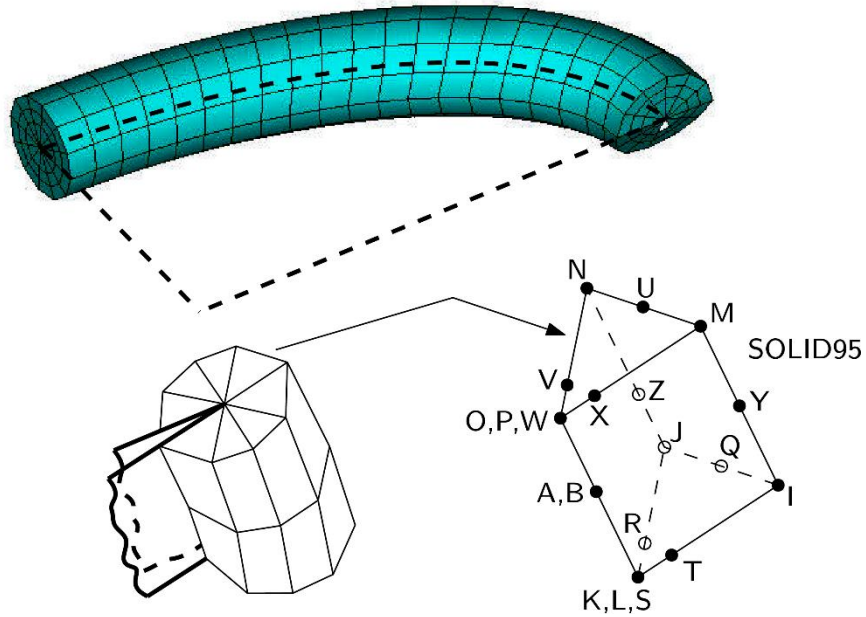
165 The plate material in FE models was assumed as linear elastic with a Young's modulus $E =$
166 200GPa and a Poisson's ratio $\nu = 0.4$.

167 a)



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169 b)



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Figure 5: a) Mesh refinement at the crack tip, b) 3D mesh at the crack tip.

172

Figure 6 shows the resulting SIF (K_I , K_{II} and K_{III}) along the front of the crack for a simple crack in a plate of $H/R=0.50$ at a distance of $R/2$ from the symmetric axis.

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Figure 6 also shows the equivalent stress intensity factor at the crack tip obtained by means of the combination of K_I , K_{II} and K_{III} according to expression (3), which assumes plane strain through the crack front.

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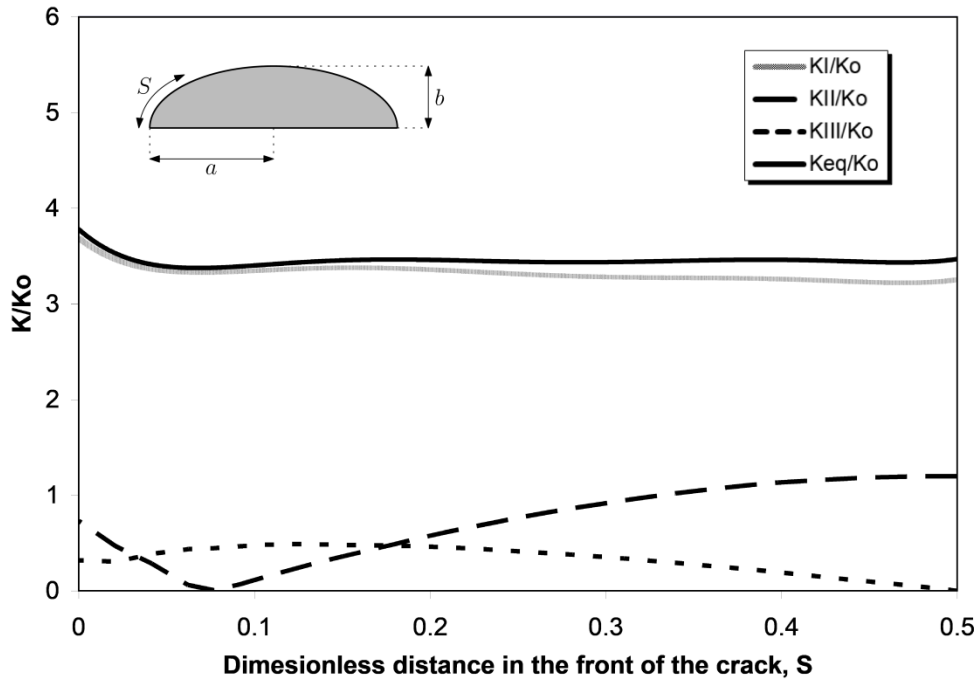
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$$K_{eq} = \sqrt{K_I^2 + K_{II}^2 + \left(\frac{K_{III}}{1-\nu}\right)^2} \quad (3)$$

178

The values of K_I , K_{II} and K_{III} as well as the distance of the crack from the external surface edge S are dimensionless, since they have been divided by K_o and the crack front length, respectively.

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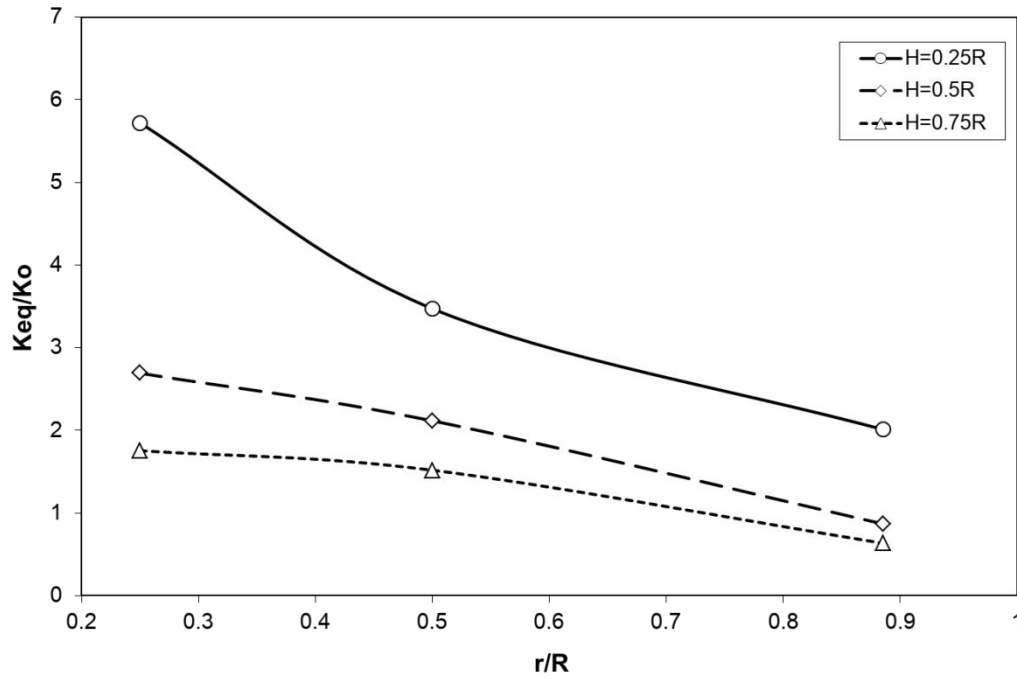


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181 Figure 6: Values of K_I , K_{II} and K_{III} along the crack front ($H/R=0.5$ with crack placed at $R/2$).

182 In this case, mode I is clearly dominant and K_{eq} is almost uniform (Iso-K) along the crack front
 183 showing a slight increment at the crack edge. Similar results were obtained for those cracks
 184 located at $r= R/2$ and $R/4$ regardless of the plate thickness H .

185 Figure 7 shows the influence of the plate thickness H and the crack location on the SIF. It
 186 displays the peak equivalent SIF along the crack front versus the dimensionless distance r/R
 187 from the crack location to the symmetry axis. These results show that damage tolerance
 188 becomes smaller in thinner plates and with cracks that are closer to the symmetry axis. When
 189 a crack enters a region under compression, it stops growing, which happens earlier in the case
 190 of thicker plates and when the initial crack is closer to the external ring support. In these cases,
 191 part of the distributed load is directly transmitted at the external support and the bending effect
 192 becomes reduced, thus relieving tensile stresses at the lower part of the disc, which is
 193 responsible for the crack propagation. This is the reason why the curves in Figure 7 become
 194 steeper when the plate becomes thinner, since the bending of the plate becomes a more
 195 preponderant mechanism, thus inducing faster crack propagation.



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Figure 7: Effects of the plate thickness and the crack location on the SIF.

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2.3. A simplified anchor head

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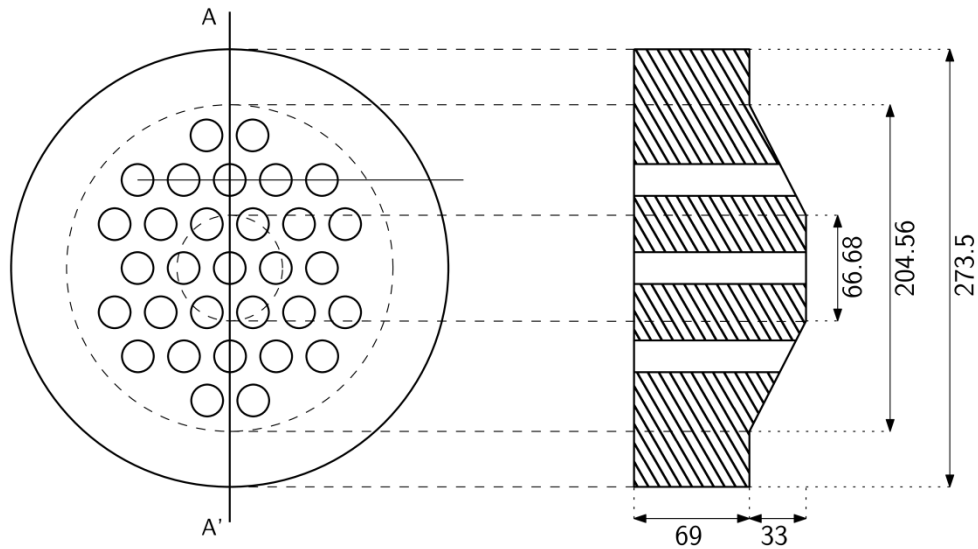
The shape of the anchor head was taken from the brochures provided by four manufacturers, with the aim of illustrating a generic pattern, instead of reproducing the specific dimensions and shape of a particular manufacturer. Figure 8 shows the geometry of the anchor head adopted in this research.

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The mechanical behaviour of an anchor head is rather complex. Since heavy prestressing loads are applied, many phenomena can be observed: the wedge-tendon gripping action, the friction at the wedge anchor head interfaces, as well as the anchor head-bearing plate interface, among others. Based on the results of previous studies [22, 23], the anchor head is simplified for the numerical analysis. The wedge holes have been suppressed and the load applied by the 31 strands has been evenly distributed on a 102.28 mm radius circle on the anchor head upper face. This circle corresponds to the shut in area by the lower radius circumference which includes all the wedge holes of the anchor head.

210



211

212

Figure 8: Sketch of the anchor head: geometry and dimensions (in mm).

213

Again, two types of cracks were taken into consideration; a circular crack and a single crack. Although a circular defect is not very realistic, the fact of making a 2D computation noticeably simplifies the task, especially the production of the finite element mesh, and allows an acceptable analysis of the influence of different variables in a quick and simple way. Furthermore, as will be seen later on, the results provided by this approach lead to a more conservative evaluation of the structural integrity.

219

2.3.1. Two-dimensional axisymmetric calculation

220

An **anchor head** of radius $1.34R$ and thickness H is considered and six values of H/R are analysed; $H/R = 0.25, 0.50, 0.67, 0.75, 1.00$ and 1.25 . A uniform load P is distributed on a circle of radius R .

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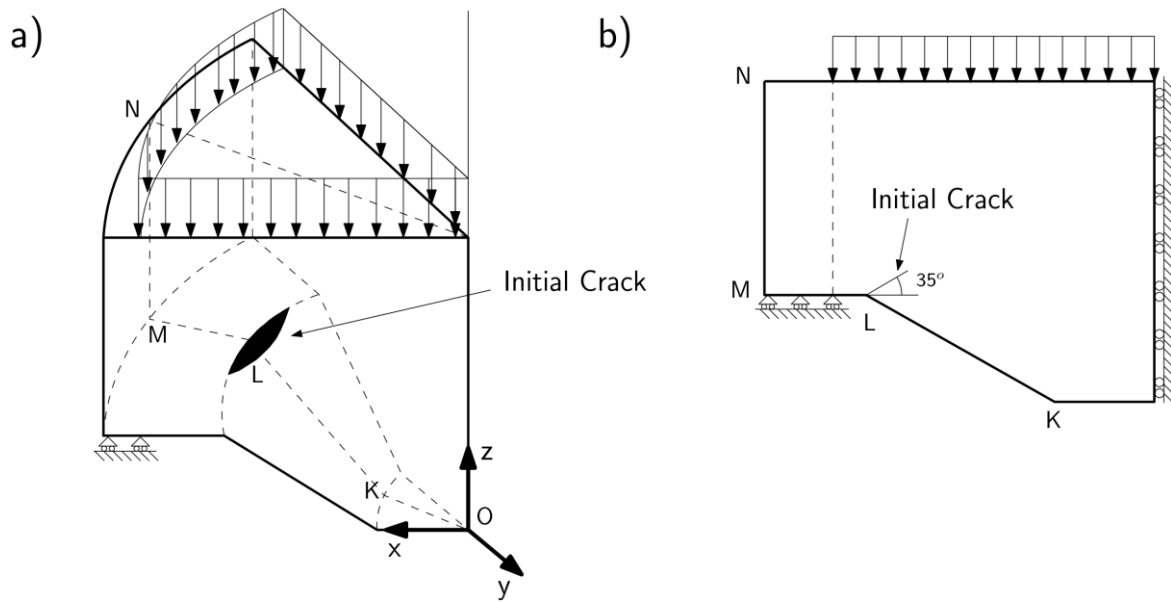
Nine different crack sizes are analysed; $a/H = 0.001, 0.002, 0.005, 0.01, 0.02, 0.05, 0.1, 0.2$ and 0.5 . Cracks were placed at the most unfavourable position and orientation which, for an anchor head of radius $1.34R$ and thickness $H/R=0.67$ corresponded to the joint zone of the anchor head with an angle of 35° with respect to the anchorage support surface, as it is shown in **Figures 9 and 10**.

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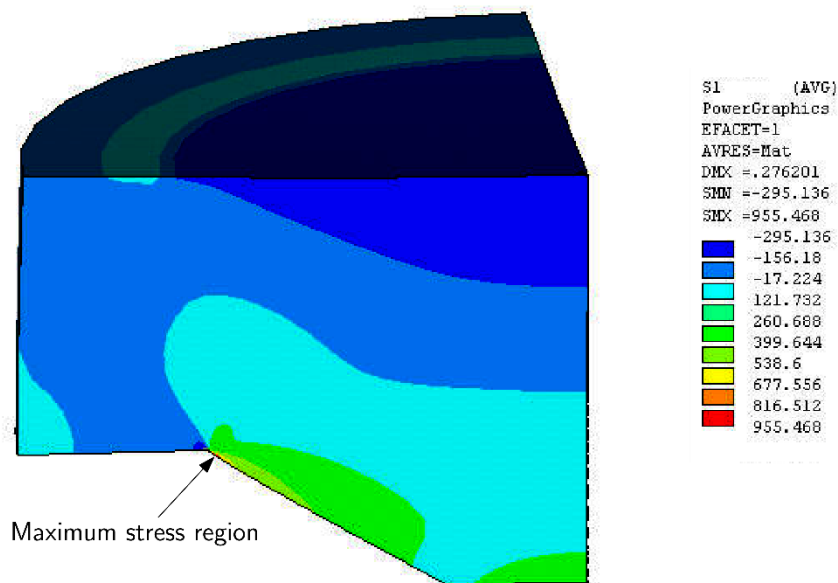
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Figure 9: Shape and location of the initial crack at the anchor head.



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Figure 10: Linear elastic finite element analysis of stresses in the anchor head.

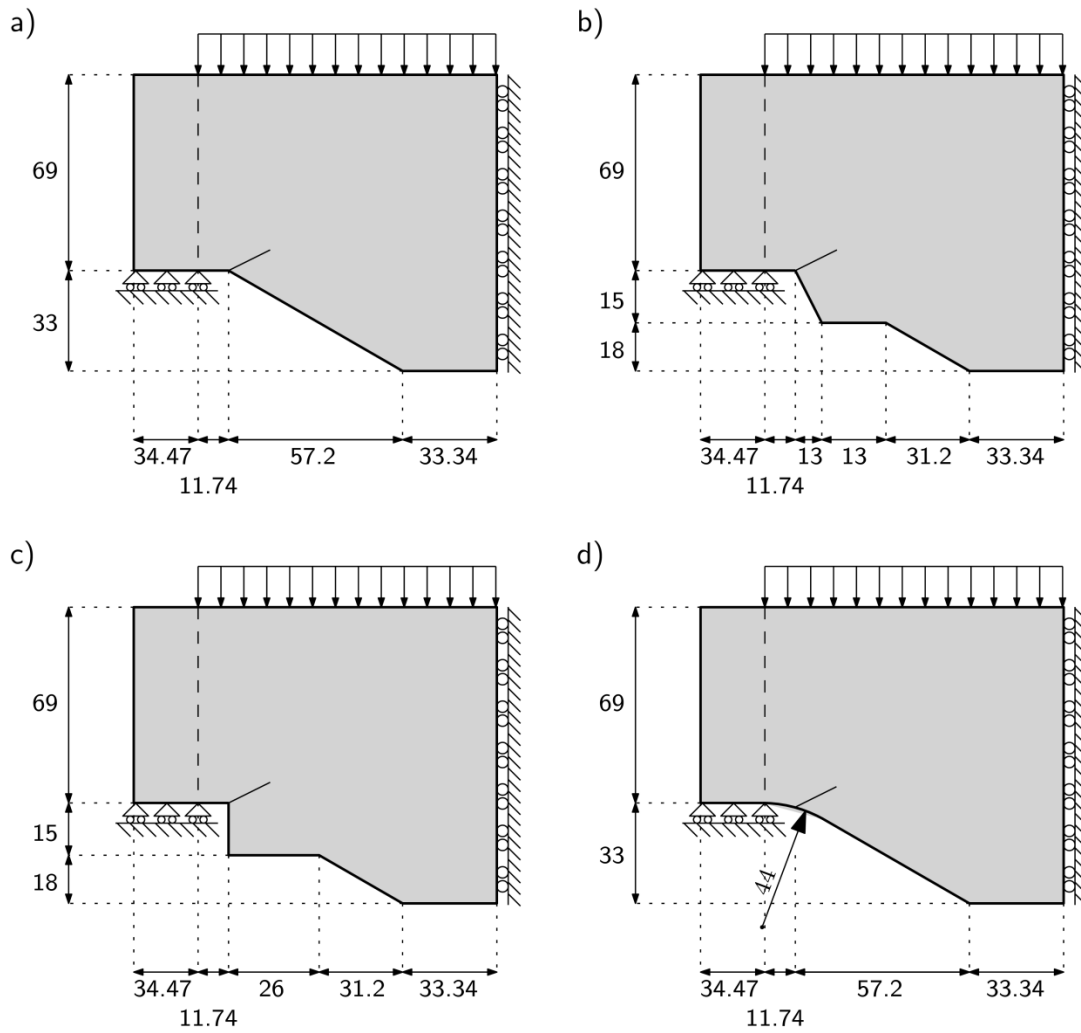
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In addition, four different **anchor head** geometries were analysed to assess the impact of the joint geometry between the cylindrical part of the anchor head and its truncated cone part on the SIF. Three different angles were studied for the joint, $\theta=90^\circ$, 130° and 150° , and a circular joint with radius of 44mm (see Figure 11).

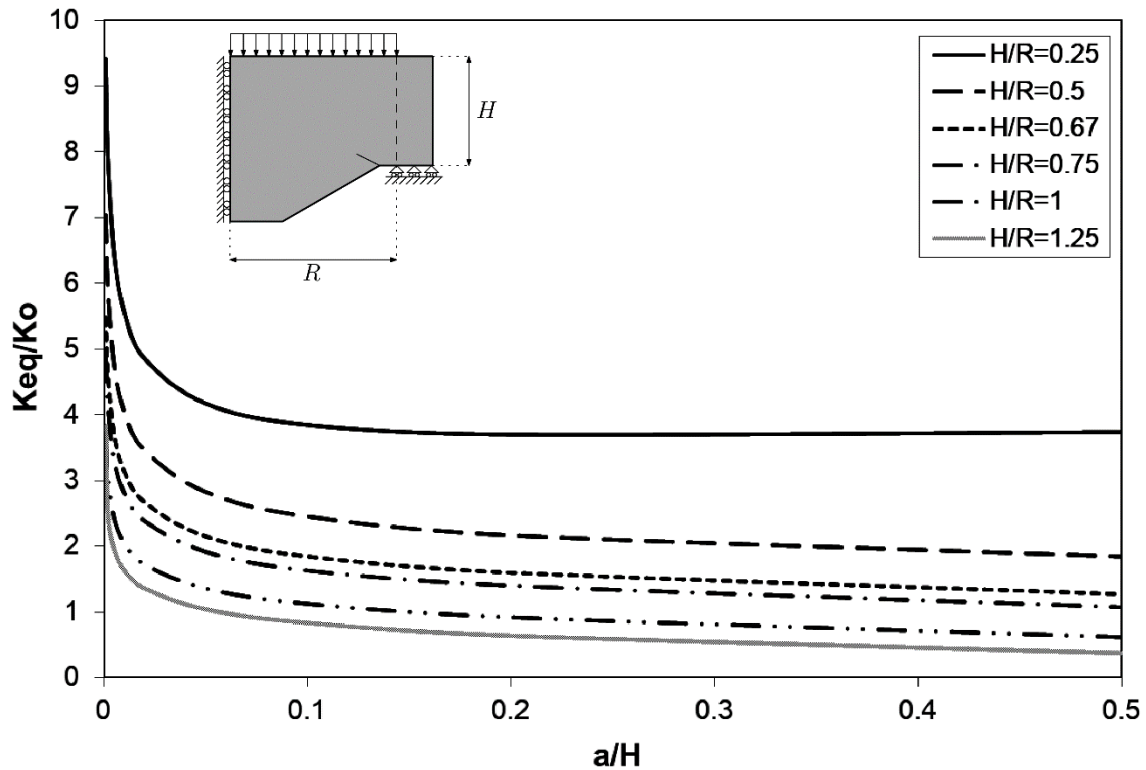


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237 Figure 11: Sketch of different joints in the anchor head analysed in this study: a) $\theta=150^\circ$, b)
 238 $\theta=130^\circ$, c) $\theta=90^\circ$ and d) circular joint.

239 The models were produced by using a similar mesh as described for the finite element model
 240 of the plate. The material was assumed as linear elastic with a Young's modulus of 200GPa
 241 and a Poisson's ratio of 0.4.

242 Figure 12 shows the effect of the **anchor head** thickness H on the SIF. This figure displays the
 243 normalized equivalent SIF against the dimensionless depth of the crack a/H for all analysed
 244 cracks.



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Figure 12: Effect of the anchor head thickness on the SIF.

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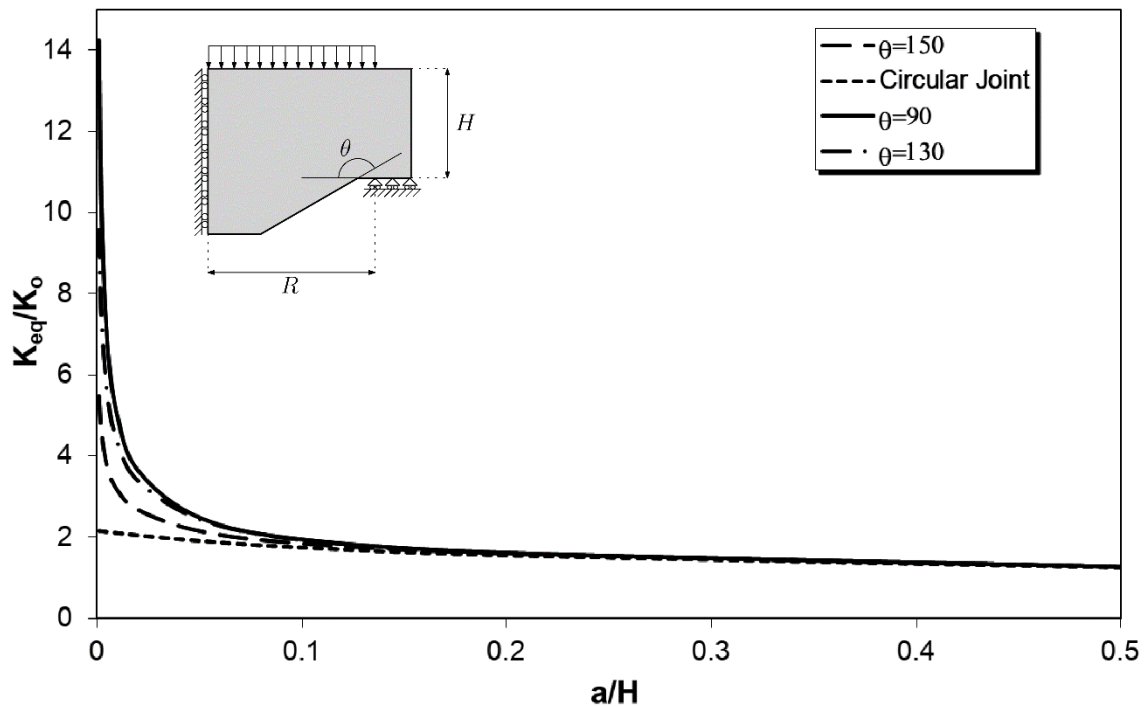
The results are consistent to those obtained with the analysis of the plate. When the anchor head thickness reduces, the damage tolerance becomes smaller, thus turning the anchorage into a more prone to damage element.

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250

Figure 13 shows the effect of the joint geometry on the SIF. It displays the normalized equivalent SIF against the dimensionless depth of the crack a/H for all analysed cracks. The effect of the joint geometry on the SIF is significant for small cracks with ratios of a/H less than 0.1; in these cases, SIF reduces when the joint angle is larger, thus increasing the damage tolerance of the anchorage head. On the other hand, the effect of the joint geometry is not relevant when cracks are big enough, since the SIF becomes similar in all cases when the a/H ratio is large enough. Therefore, the effect of the joint shape has a great influence on the damage tolerance of the element under small defects; when the joint angle is larger (see Figure 11a), the stress raiser is reduced in comparison with smaller angles (see Figure 11c), which, as shown in Figure 13, has a strong influence on the value of the SIF, thus leading to more damage tolerant elements. If the thickness transition is soft, in especial, if a circular transition is employed (see Figure 11d), damage tolerance is highly increased in the case of very small cracks as observed in Figure 13.

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Figure 13: Effect of the joint geometry of the anchor head on the SIF

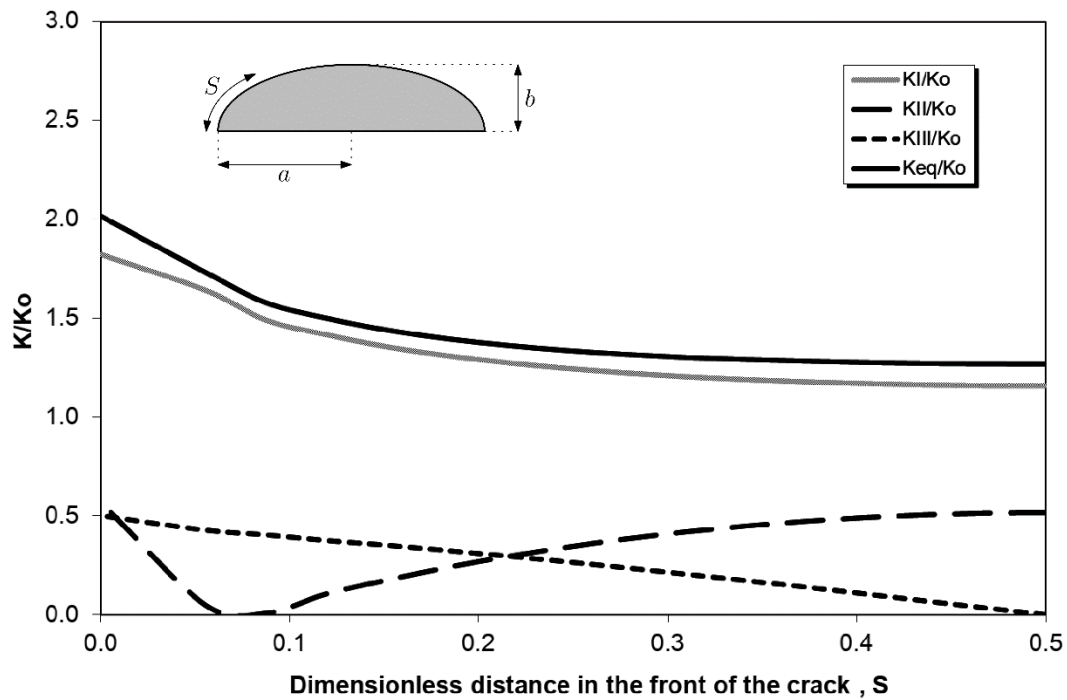
265 *2.3.2. Three-dimensional calculation*

266 For the single crack, a 3D computation was needed. An anchorage of radius $1.34R$ and
 267 thickness H is considered. The value of the **anchor head** thickness H was set to 69 mm, which
 268 results in a H/R ratio of 0.67. A uniform load P is distributed on a circle of radius R .

269 The crack had a semi-elliptical shape, with a minor semi axis a and a major semi axis b (see
 270 Fig. 14) and five relations b/a were studied: 1, 1.3, 2, 3 and 5, with six values of crack depth
 271 a : 1, 2, 4, 6, 8 and 10 mm. The relation b/a equal to 1.3 was studied because it corresponds to
 272 the average measured dimensions of the cracks induced in the experimentally tested PMMA
 273 specimens.

274 Figure 14 shows the resulting normalized SIF (K_I , K_{II} , K_{III} and K_{eq}) along the crack front for
 275 a simple crack of 1 mm depth and a relation b/a of 1.

276 For this particular case, mode I is clearly dominant along the crack front. The peak SIF resulted
 277 at the surface edge. Similar results were obtained for those cracks with low relations of b/a .



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Figure 14: Values of K_I , K_{II} and K_{III} along the crack front ($a=1$ mm, $b/a=1$).

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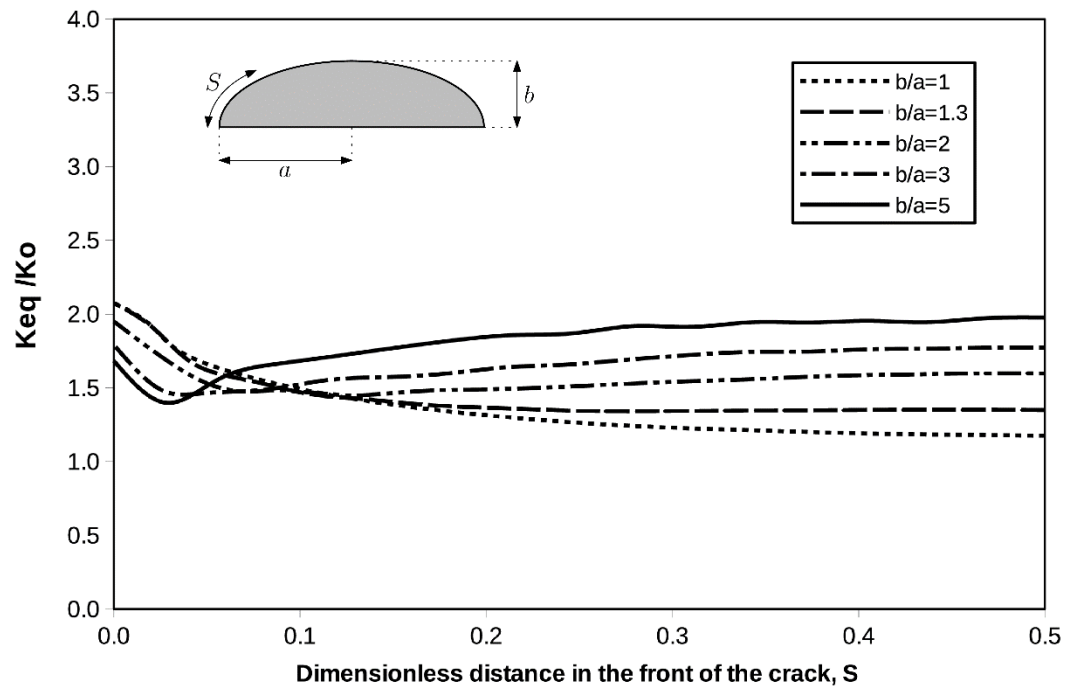
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Figure 15 shows the equivalent SIF along the front of the crack for a 2mm deep crack and different relations of b/a . In this case, relation $b/a = 2$ approximately leads to a value of an iso- K in the front of the crack. The peak SIF for high relations of b/a shifts from the surface edge to the deepest location of the crack front. Analogous results were obtained with the other crack depths.



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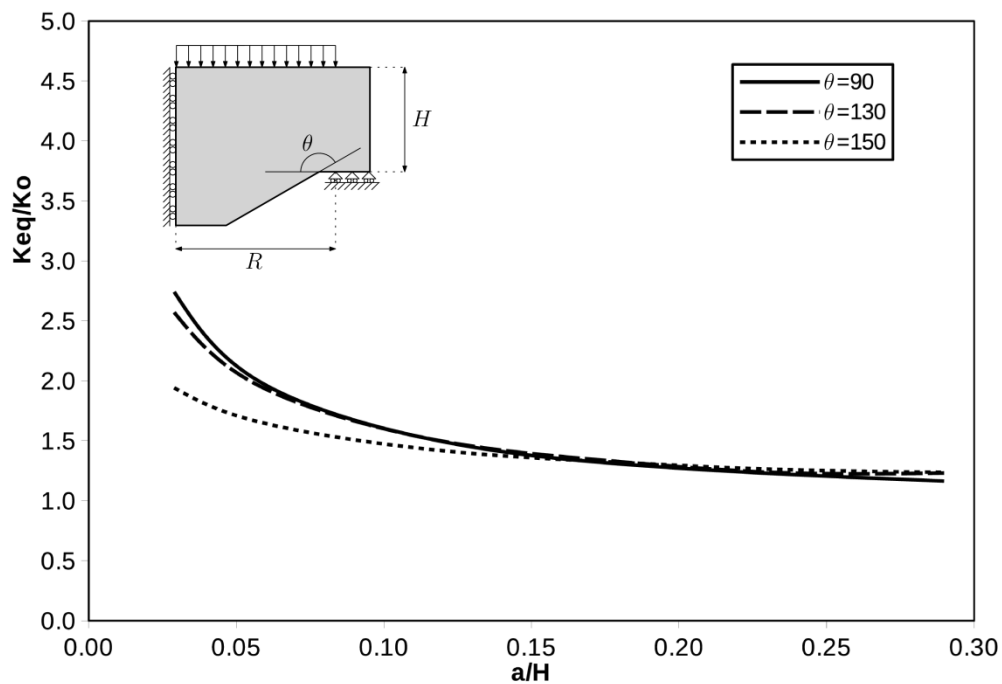
Figure 15: Effect of b/a ratio on the SIF along crack front ($a=2$ mm)

287 As in the 2D analysis, three different anchor head geometries were analysed to assess the
288 impact of the joint geometry between the cylindrical part of the anchor head and its truncated
289 cone part on the SIF. Three different angles were studied for the joint, $\theta=90^\circ$, 130° and 150° .

290 For each geometry, a different b/a ratio was used. A value of 1.4 was used for those anchorages
291 with a joint of 90° , 1.5 and 1.3 for those anchorages with angles of 130° and 150° respectively.
292 These values were used because they are the average measured values of crack dimensions
293 induced in experimentally tested PMMA specimens.

294 Figure 16 shows the effect of the joint geometry on the SIF. It displays the peak normalized
295 equivalent SIF along the crack front against the dimensionless depth of the crack a/H for all
296 analysed cracks.

297 The results are analogous to those obtained with the 2D analysis. In the case of small cracks,
298 the equivalent SIF increases when the θ angle is closer to 90° and decreases when it becomes
299 larger, thus increasing the damage tolerance of the element. This is due to the better
300 transmission of stresses at the change of thickness. An abrupt change of thickness, such as that
301 induced by $\theta=90^\circ$, leads to a high stress concentration around the crack, which eases crack
302 propagation. When θ becomes larger and the change of thickness is progressive, the stress
303 concentration is reduced.



304

305

Figure 16: Effect of the joint geometry of the anchor head on the SIF

306

307 **3. Experimental work**

308 To check the extent and accuracy of the previous numerical approach, an experimental program
309 was carried out. Several **anchor heads**—some with a disc shape and others with more realistic
310 shapes— were damaged with single cracks and tested until failure.

311 **3.1. Material characterisation**

312 The material chosen for the experimental programme was polymethyl-methacrylate (PMMA),
313 an amorphous glassy polymer that exhibits a quasi-elastic linear behaviour at room
314 temperature, and because of its excellent optical properties, it allows controlling the crack
315 generation process as well as its progression during the test.

316 Tensile tests, with cylindrical samples, and fracture toughness tests, with notched beams, were
317 performed according to standards [24-27] **as described in [17]**. The average mechanical
318 properties are shown in Table 1.

319 Table 1: Average mechanical properties of PMMA

UTS(MPa)	E(GPa)	ν	K_{IC} (MPa m ^{1/2})
76.6	2.66	0.42	1.25

320

321 **3.2. Samples and experimental equipment**

322 Two specimen geometries were tested: circular discs and simplified **anchor heads**. Shapes and
323 dimensions are shown in Figures 17 and 18.

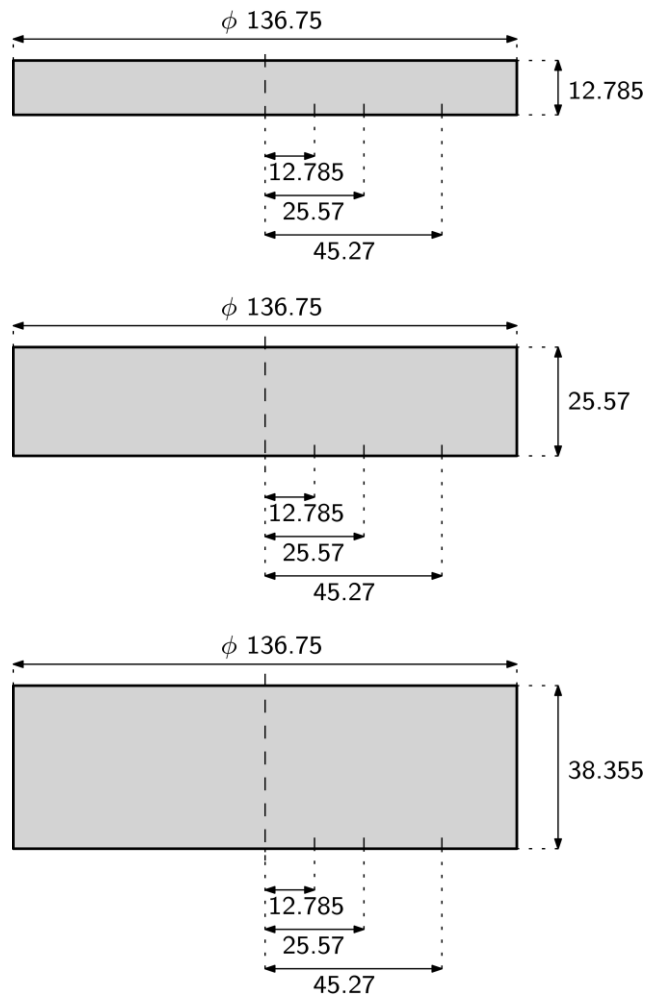
324 Both specimen types were manufactured using **1 m** long PMMA bars with a diameter of 140
325 mm. **Dimensions of the experimental samples were scaled using a ratio of 1:2 compared with**
326 **the finite element models in order to adjust them to those of the PMMA bars.**

327 For each anchorage, the crack was generated in two stages; first, a notch was cut with a steel
328 disc of 25 mm diameter and 0.5 mm thickness. In a second step, a crack was made inserting a
329 sharp circular blade and pressing in a controlled way. This wedging effect generates cracks
330 with almost elliptical shapes, which was verified after the tests.

331 The testing of circular discs was carried out to assess the effect of the plate thickness H and the
332 location of the crack on the damage tolerance of the anchor head. Single cracks were placed in
333 circular discs at three different locations at distance r from the symmetry axis. Single cracks of
334 similar dimensions were placed with its plane normal to the anchorage support surface. Three
335 sets of six circular plates were manufactured combining three thicknesses, 12.785, 25.57 and
336 38.355 mm, and three crack locations, $r = 12.785, 25.56$ and 45.27 mm.

337 **The analysis of different anchor head geometries shown in Figure 11 suggest that the angle θ**
338 **(see Figure 13) may play a relevant role in the fracture behaviour of the anchor head.** Three
339 sets of six anchorages were prepared in order to assess the effect of the joint geometry between
340 the cylindrical part of the anchor head and its truncated cone part on the damage tolerance; one
341 with $\theta = 90^\circ$, (anchorage type A90), one with $\theta = 130^\circ$, (anchorage type A130) and another one

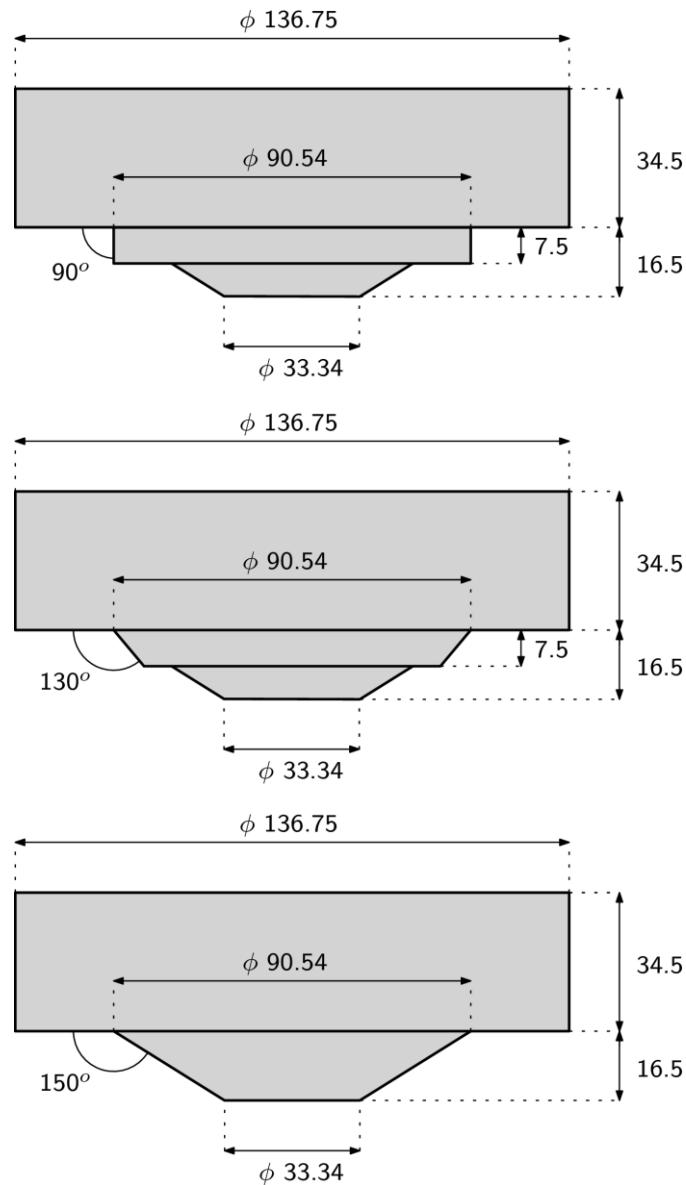
342 with $\theta = 150^\circ$ (anchorage type A150). Single cracks of different sizes were placed in the most
343 unfavourable zone with its plane oriented 35° with respect to the anchorage support surface.



344

345

Figure 17: Circular plate sample geometries (distances in mm).



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Figure 18: Anchorage sample geometries (distances in mm).

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The experimental setup is shown in Figure 19. The anchor head was supported on an external ring to reproduce the boundary conditions used in the numerical analysis; this boundary was lubricated to diminish the friction forces; it must be noted that friction could make results more similar to actual on-site cases, but in this study lubrication permits easier comparison with the numerical results. The load actuator applied a uniform pressure upon a circular surface on the specimen. An elastomeric disc of 10 mm thickness was placed between the actuator and the anchorage to evenly distribute the load.

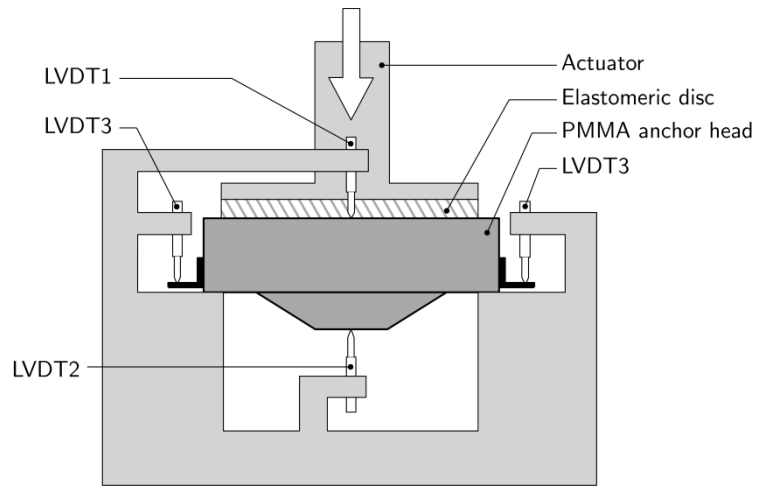
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During the test, the applied load, the vertical displacement of the load application point (extensometer LVDT 1), the vertical displacement of the anchorage lower face central point (LVDT 2) and the possible lifting of the anchorage edges in the support, measured in two diametrically opposed points (extensometers LVDT 3), were continuously recorded.



359 Figure 19: Experimental setup.

360

361 **3.3. Experimental results**

362 Tests were made under displacement control of the load actuator with a speed of 0.4 mm/min.
 363 During the test, the extensometers LVDT 3 in Figure 14 did not detect any movement, as was
 364 hypothesized in the numerical simulation.

365 The initial crack dimensions were measured at the fractured samples after the tests; these values
 366 are presented in Tables 2 and 3.

367 Table 2: Circular plate geometry and crack dimensions measurements.

Sample	H (mm)	Crack location r (mm)	a (mm)	$2b$ (mm)	b/a
H12.1	12.8	46.7	4.90	30.00	3.06
H12.2			6.14	28.40	2.31
H12.3		34.0	4.90	28.80	2.86
H12.4			4.82	30.40	3.15
H12.5		12.2	4.40	29.02	3.30
H12.6			5.94	30.00	2.53
H25.1	25.6	47.2	7.60	29.50	1.94
H25.2			7.20	30.50	2.12
H25.3		24.2	5.46	29.30	2.68
H25.4			6.00	30.30	2.53
H25.5		12.2	4.86	29.60	3.05
H25.6			5.14	29.10	2.83
H38.1	38.3	43.8	6.22	29.76	2.39

H38.2			6.18	26.20	2.12
H38.3		23.5	6.30	30.04	2.38
H38.4			5.72	29.60	2.59
H38.5		11.6	5.64	31.10	2.76
H38.6			5.00	30.10	3.01

368

369

Table 3: Crack dimensions measurements in anchor heads.

Sample with $\theta = 150^\circ$	a (mm)	b (mm)	b/a
A90.P1	3.18	5.16	1.62
A90.P2	4.29	5.70	1.33
A90.P3	4.42	6.64	1.50
A90.P4	4.69	6.64	1.42
A90.P5	6.20	8.60	1.39
A90.P6	6.75	9.15	1.36
A130.P1	3.49	5.44	1.56
A130.P2	3.48	5.43	1.56
A130.P3	4.69	6.99	1.49
A130.P4	4.20	6.81	1.62
A130.P5	5.01	7.36	1.47
A150.P1	3.33	4.36	1.31
A150.P2	3.44	4.83	1.41
A150.P3	4.43	6.27	1.42
A150.P4	4.71	6.43	1.36
A150.P5	4.79	6.08	1.27
A150.P6	6.10	7.01	1.15

370

371 The ultimate load was recorded for every single test. Ultimate loads for the two sets of samples
 372 are drawn in Tables 4 and 5; the crack depth of every sample is also provided in order to
 373 facilitate the analysis of the experimental results.

374

Table 4: Ultimate loads for circular plate samples.

Sample	Crack depth (mm)	Ultimate load (KN)
H12.1	4.90	57.0
H12.2	6.14	59.0
H12.3	4.90	27.1
H12.4	4.82	28.4
H12.5	4.40	20.9
H12.6	5.94	20.1
H25.1	7.60	101.4

H25.2	7.20	106.0
H25.3	5.46	36.3
H25.4	6.00	35.1
H25.5	4.86	30.7
H25.6	5.14	31.4
H38.1	6.22	182.6
H38.2	6.18	185.0
H38.3	6.30	69.2
H38.4	5.72	70.7
H38.5	5.64	58.2
H38.6	5.00	59.5

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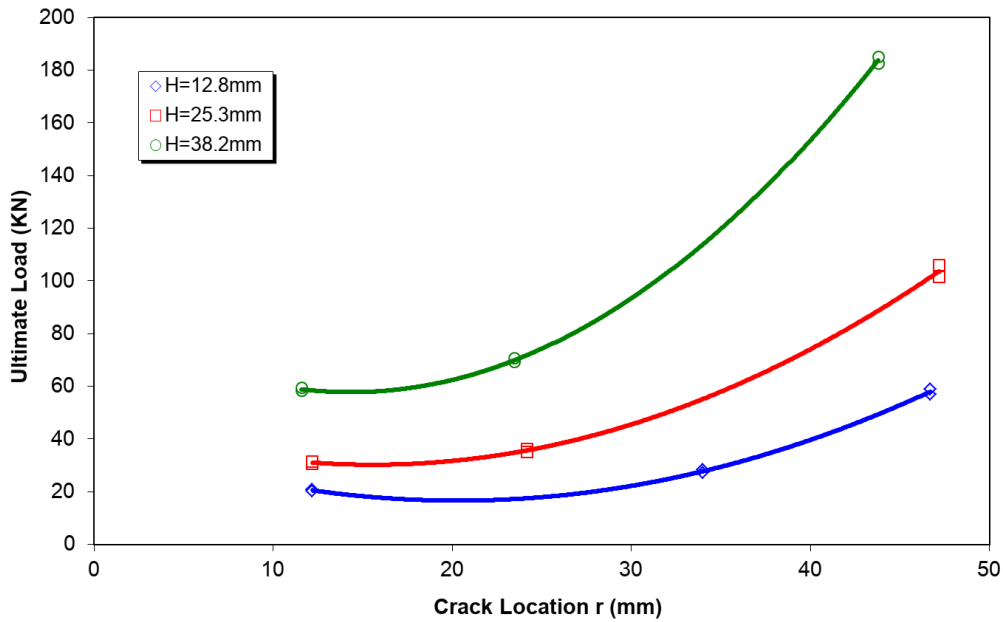
376

Table 5: Ultimate loads for anchor head samples.

Sample	Crack depth(mm)	Ultimate load (KN)
A90.P1	3.18	95.0
A90.P2	4.29	75.5
A90.P3	4.42	88.7
A90.P4	4.69	72.6
A90.P5	6.20	78.0
A90.P6	6.75	76.0
A130.P1	3.49	99.06
A130.P2	3.48	128.1
A130.P3	4.69	109.54
A130.P4	4.20	106.47
A130.P5	5.00	129.61
A150.P1	3.33	118
A150.P2	3.44	134.8
A150.P3	4.43	109.6
A150.P4	4.71	118.0
A150.P5	4.79	130.0
A150.P6	6.10	105.0

377

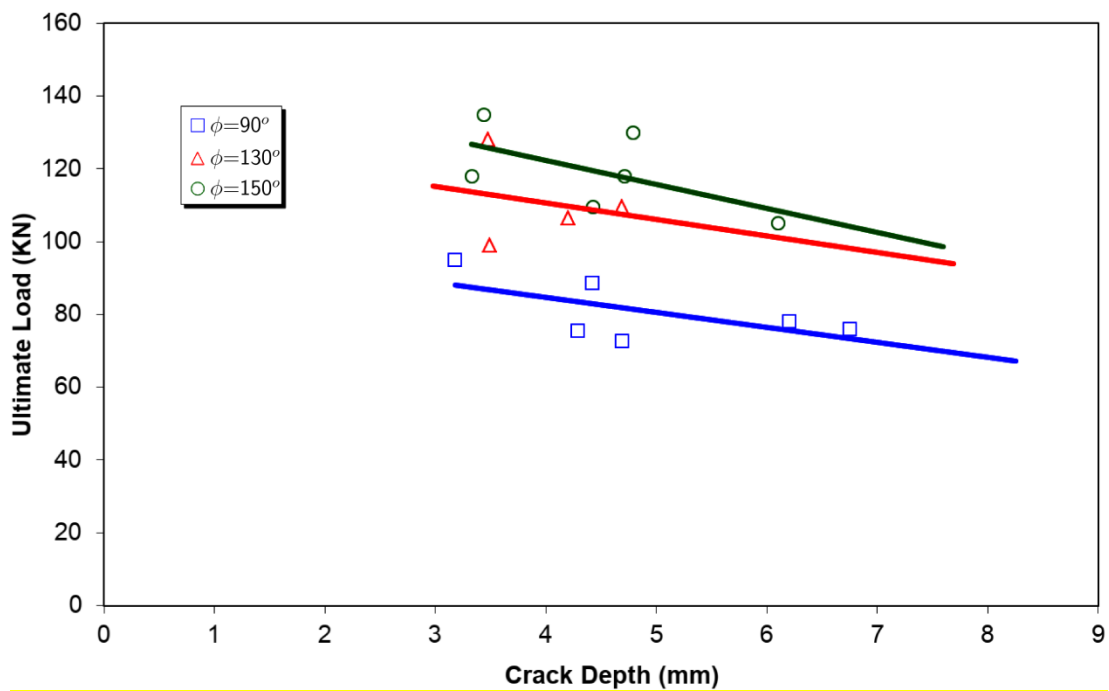
378 Figure 20 shows the influence of plate thickness H and the location of the crack; as the
379 thickness of the circular plate increases, breaking load increases, as expected. The influence of
380 crack locations - through distance r from the symmetry axis - is clearly observed, since breaking
381 loads in those cases with cracks closer to the symmetry axis are much lower than those with
382 cracks located further.



383

384 Figure 20: **Effects** of thickness and crack location on ultimate loads in circular plate samples.

385 Figure 21 shows the influence of crack depth on the **breaking loads** of **anchor head** samples; as
 386 the crack size increases, **breaking loads** —on average— decrease. The influence of **anchor head**
 387 shape —through angle θ — is clearly observed, as **breaking loads** for sharp transitions are much
 388 lower than for smooth ones. This can be explained in terms of stress concentration at the change
 389 of the plate thickness, as mentioned before, which is more abrupt when $\theta=90^\circ$ and more
 390 progressive when $\theta=150^\circ$.



391

392 Figure 21: **Effects** of joint geometry and crack depth on ultimate loads in anchorage samples.

393 **3.4. Comparison with numerical predictions**

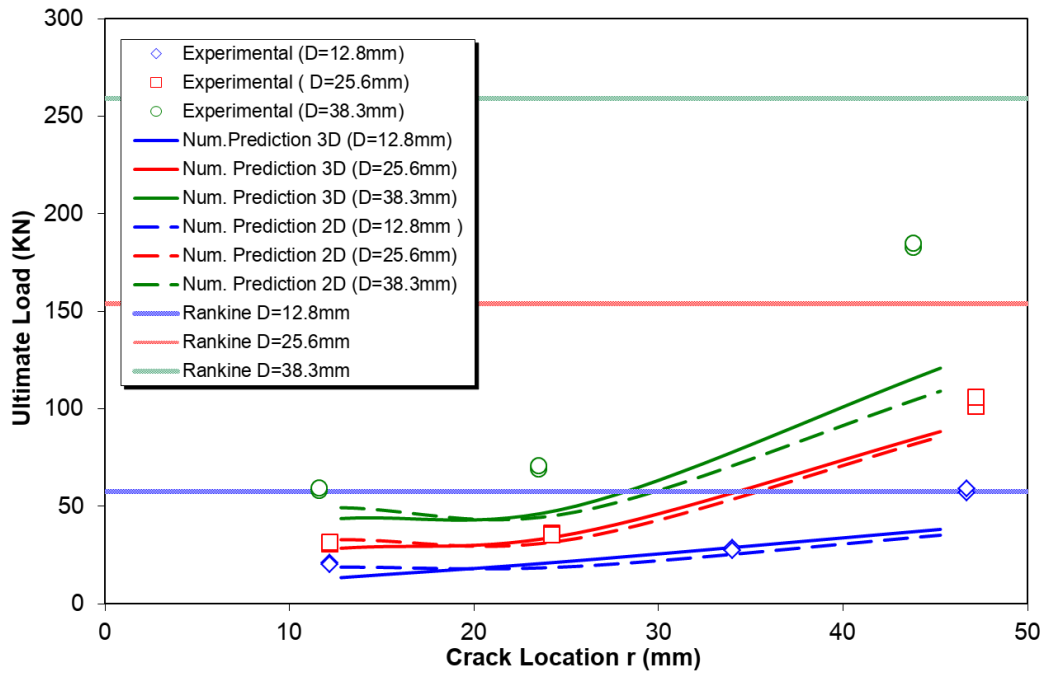
394 Predicted **breaking loads** from the numerical models were derived from the resulting stress
395 intensity factors by means of a local fracture criterion by matching the equivalent stress
396 intensity factor with the experimentally obtained fracture toughness:

397
$$K_{eq} = \sqrt{K_I^2 + K_{II}^2 + \left(\frac{K_{III}}{1-\nu}\right)^2} = K_{IC} \quad (3)$$

398 Figure 22 shows the fit of the experimental **breaking loads** with the predictions derived from
399 the 2D and 3D finite element models of circular plates. As shown in Figure 22, the fit of the
400 experimental points with the numerical predictions is good for plates of 12.8 and 25.6mm
401 thickness and with cracks located closer to the axisymmetric axis ($r=R/2$ & $R/4$). However, the
402 fit of experimental points is not so good in the case of thicker plates, $H=38.6$ mm, with cracks
403 located further from the axisymmetric axis ($r=0.88R$).

404 The influence of defects on the performance of the plates can be roughly evaluated by
405 comparing the experimental values of a cracked plate with an estimation of the **breaking load**
406 of a plate without defects. Using finite element models of circular plates without cracks and a
407 fracture criterion based on the maximum stress (Rankine criterion), the **breaking load** was
408 estimated for each circular plate. Figure 22 also shows the **breaking loads** of defect-free plates
409 which were derived using these 2D models. As can be observed, the experimental **breaking**
410 **load** for the thinner ($H=12.8$ mm) and with a crack at the furthest studied location ($r=0.88R$) is
411 close to the value predicted by Rankine criterion. For the thicker plates, the experimental values
412 are much lower than those predicted by Rankine criterion.

413 Numerical models predict reasonably well the effect of plate thickness and the crack location,
414 since **breaking loads** agree with the experimental results. It should be noted that no significant
415 discrepancies were found between the 2D and 3D **breaking load** predictions, therefore, 2D
416 models can be recommended in axisymmetric cases, since they result in much simpler to build
417 models and easier to interpret results.



418

419

Figure 22: Fit of experimental **breaking loads** to predictions for circular plates.

420

Figure 23 shows the fit of the experimental **breaking loads** with the numerical predictions

421

derived from the results obtained with the finite element models of **anchor heads**. As shown in

422

Figure 23 the fit of the experimental results with the numerical predictions is not very good for

423

the three different **anchor head** samples which were tested.

424

Significant discrepancies were also found between the 2D and 3D numerical predictions of the

425

breaking load. Numerical predictions derived from two dimensional axisymmetric models

426

were found conservative.

427

Figure 23 shows the theoretical **breaking loads** of defect-free anchorages which were derived

428

by means of the Rankine failure criterion from 2D models of defect-free anchorages. As can

429

be observed, the experimental values of the three tested sample geometries are much lower

430

than those predicted by the Rankine criterion.

431

Numerical models predict reasonably well the effect of the crack size and the joint geometry

432

on the **breaking load** of anchorage samples. Numerical results predict a decrease of the **breaking**

433

load when the crack depth increases. The slope of the curves shown in Figure 23 is flatter for

434

the range of tested crack sizes than that for smaller cracks with depths below 1mm. Hence, the

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effect of the tested crack sizes on **breaking load** is less significant than the effect expected for

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cracks with depths below 1 mm. However, numerical results derived from the 2D and 3D

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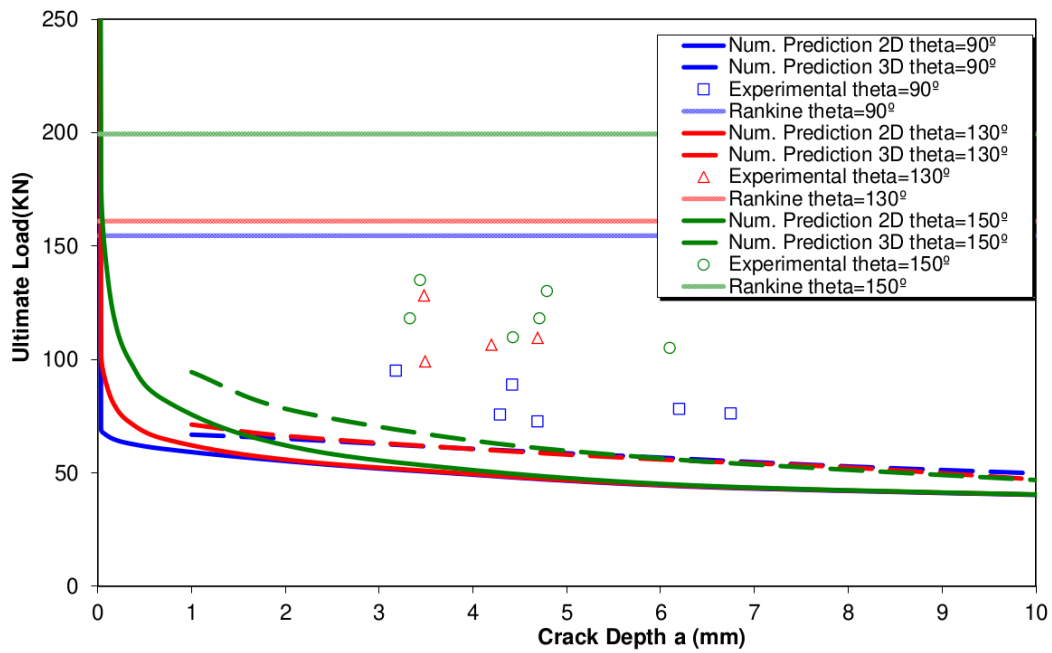
models predict no differences on **breaking loads** between the different joint geometries for

438

cracks with depths around 4 and 5 mm. This prediction is not consistent with the test results on

439

which differences in **breaking loads** were found for the whole range of tested crack sizes.



440

441

Figure 23: Fit of experimental **breaking loads** to predictions for anchorages.

442

These results are consistent with those obtained for circular plates, because the thickness and crack location of the anchorage samples are similar to those of the circular plate samples which showed the more deficient fit with the experimental results.

443

444

445

Damage location on **anchor head** specimens is very close to the contact face between the specimen and the supporting annular ring, therefore the boundary conditions applied on these locations could have an impact on the results. In the finite element models, the nodal displacement at contact faces were restricted in the normal direction and no restrictions were applied in other directions.

446

447

In order to better reproduce the test conditions, the supporting ring was also considered into the 2D finite element models. The contact between the specimen sample and the supporting ring was modelled by means of contact elements at the interface assuming a friction coefficient of 0.5 between PMMA and steel which is in line with the values found in literature [28].

448

449

450

In the case of 3D models, in order to simplify their performance and avoid numerical problems, instead of modelling the contact between the specimen and the supporting ring, the displacements resulting from the 2D analysis were used as input in the 3D models of damaged specimens. This allowed calculating the stress intensity factors to derive the **breaking loads** for the range of analysed crack sizes.

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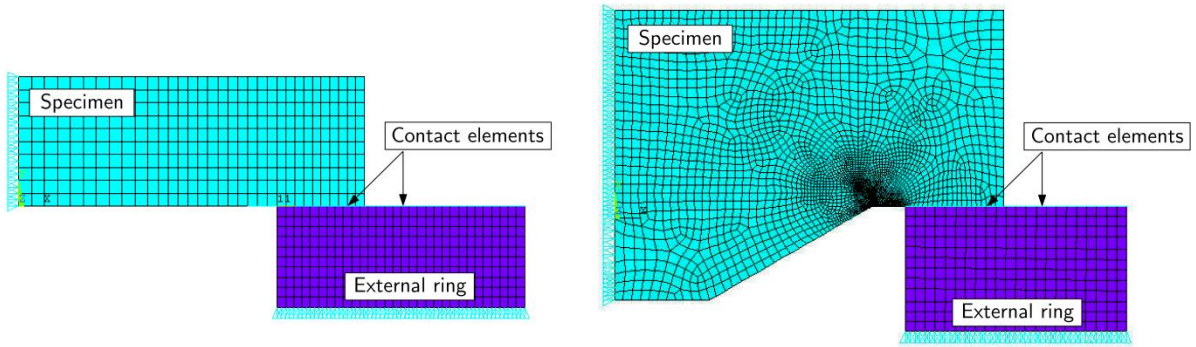
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Figure 24: 2D finite element models including the effect of friction at the interface.

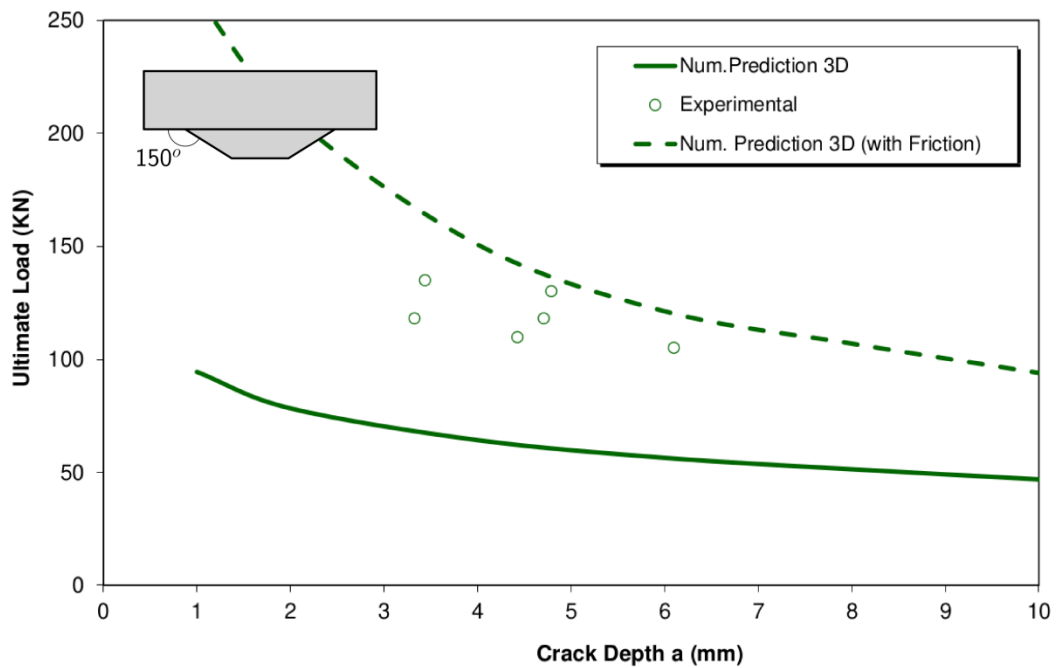
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Figures 24 and 25 show the influence of friction on the breaking loads of anchor head and circular plate samples. The experimental breaking loads of anchorage samples are placed between both predictions with and without friction.

462

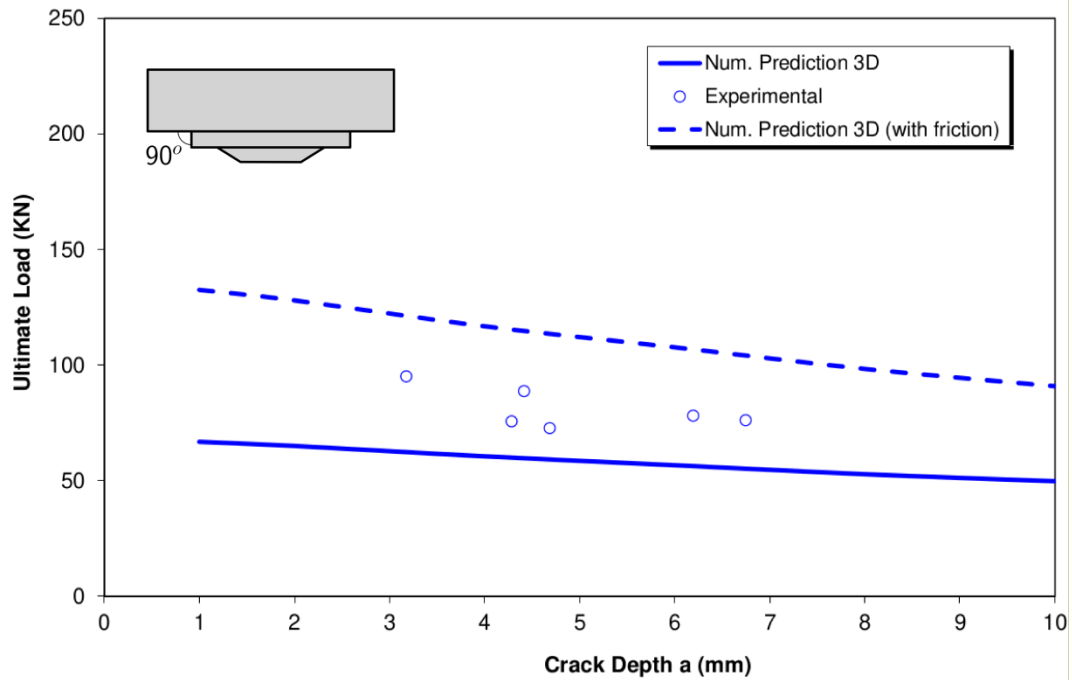
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464 a)



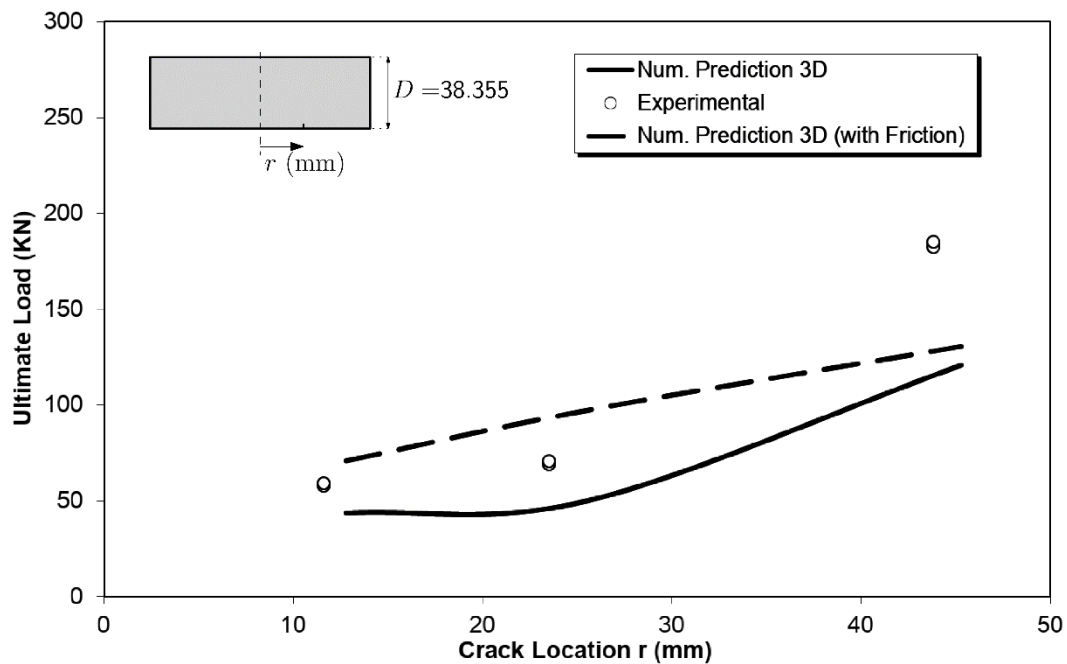
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466 b)



467

468 Figure 25: Effect of friction on breaking loads in anchorage specimens: a) specimens with
 469 $\theta=150^\circ$, b) specimens with $\theta=90^\circ$.



470

471 Figure 26: Effect of friction on breaking loads in circular plate specimens ($D=38.3\text{mm}$)

472 For circular plates, the breaking load curve including friction effects is above the experimental
 473 values for those cracks located at distances of $R/2$ and $R/4$ from the symmetry axis; however
 474 the curve is below the experimental breaking load for those cracks placed at the furthest
 475 location from the axis. The reason of this is that the stress intensity factor in mode I is negative
 476 due to the compressive stress field around the crack when the defect is close to the external
 477 support ring, therefore the fracture is driven by a combination of mode I and mode II.

478 The predicted **breaking loads** for those cracks were consequently derived assuming K_I to be
 479 zero in the fracture criterion. This approach is just a rough estimation because the 3D models
 480 used in the present work are not suitable to work with contact and friction effects between crack
 481 faces under a compressive stress field and therefore the predicted **breaking loads** should be
 482 carefully considered.

483 **4. A damaged anchor head in an aggressive environment**

484
 485 This section presents an example that deals with a commercial anchor head with a small
 486 damage produced during machining, transport or placement. This damage is, in general,
 487 accepted because it fulfils all standards of geometry, chemical composition, non-corrosion, and
 488 nominal strength. This damage can be idealized, from the safe side, as a tiny crack (usually of
 489 tenths of a millimetre, difficult to detect during a routine control test) placed at the most critical
 490 zone.

491 A simplified anchor head geometry was selected with the purpose not to show up any
 492 commercial anchorage. A crack was placed in the transition region where tensile stresses reach
 493 maximum values. Stress intensity values for these cracks were previously obtained and are
 494 available in [17] and [29].

495 Table 6 summarises the mechanical properties of two materials used for manufacturing anchor
 496 heads, steel and cast iron. These properties were measured experimentally by means of
 497 standardised tests. Two values of the fracture toughness are given, since depending on the
 498 environmental conditions this parameter can be reduced, with a significant impact on the
 499 fracture behaviour of the element. The values of K_{ISCC} correspond to the stress-corrosion-
 500 cracking threshold behaviour in synthetic sea water, according to ASTM D1141-52 [30, 31].
 501 In the case of steel, both values of K_{IC} are over $100 \text{ MPa}\cdot\text{m}^{1/2}$, but in the case of cast iron, the
 502 value reduces from 60 to $35 \text{ MPa}\cdot\text{m}^{1/2}$ under aggressive environmental conditions. Regarding
 503 the rest of the mechanical properties, both materials have acceptable strength and ductility
 504 according to usual design rules.

505 **Table 6: Mechanical properties of steel and cast iron anchorages.**

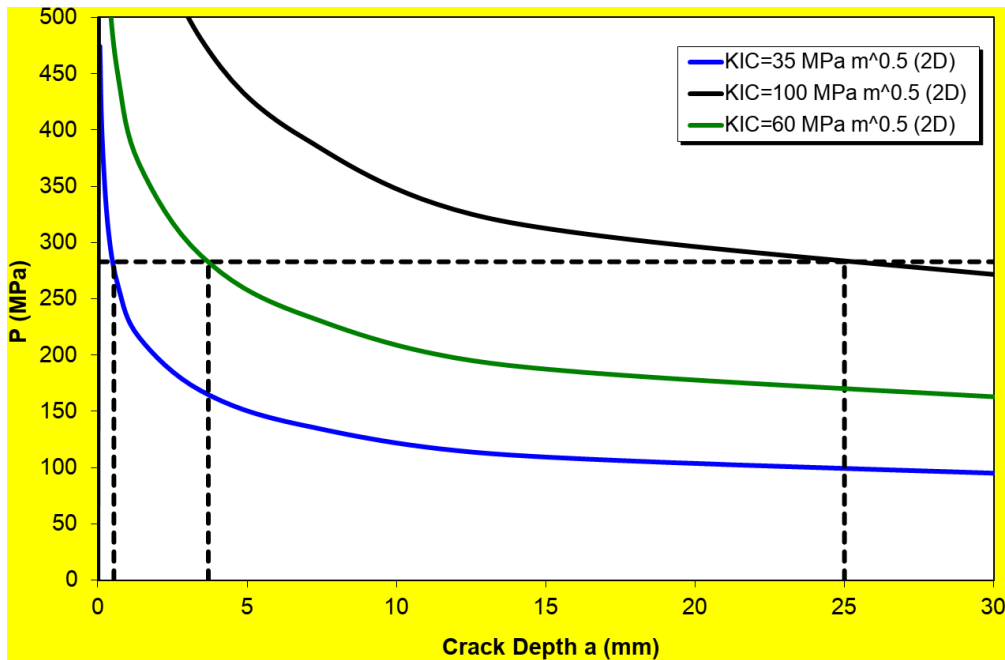
	E (GPa)	$\sigma_{0.2}$ (MPa)	σ_{max} (MPa)	ϵ_m (%)	K_{IC} ($\text{MPa}\cdot\text{m}^{1/2}$)	K_{ISCC}^* ($\text{MPa}\cdot\text{m}^{1/2}$)
Steel	210	680	850	9	> 100	> 100
Cast iron	163	850	1100	5	60	35

506 ϵ_m = strain under maximum load

507 * Stress-corrosion-cracking threshold behaviour in synthetic sea water, according to ASTM D1141-52 [30]

508 Figure 27 shows the damage tolerance diagram for the considered anchor head. A steel
 509 anchorage, as the one reported in Table 6, will behave according to the line labelled with K_{IC}
 510 = $100 \text{ MPa}\cdot\text{m}^{1/2}$. A cast iron anchorage, with properties shown in Table 6, will perform
 511 according curves labelled $60 \text{ MPa}\cdot\text{m}^{1/2}$ or $35 \text{ MPa}\cdot\text{m}^{1/2}$, depending on the environmental
 512 conditions; in this case, the aggressive environment was sea water [31].

513 Under a design stress of 280 MPa, which is a standard value for the yield stress of a structural
514 steel, the length of the critical crack that triggers brittle fracture is, according to Figure 27,
515 about 25 mm for steel, 3.7 mm for cast iron under non-aggressive environment, or 0.54 mm in
516 presence of sea water. As can be noticed, when the anchorage is not manufactured with the
517 suitable material, a very small crack —generated during fabrication or grown by fatigue or
518 corrosion— can become catastrophic.



519

520 Figure 27: Design stress depending on anchorage material and environment.

521 5. Final comments and conclusions

522

523 This research has shown the role of defects on the loading capacity of structural components
524 and how using the concept of damage tolerance can help in their structural design. The paper
525 ends with an example, taken from actual anchor heads.

526 **Breaking loads** for circular transversally loaded discs and **anchor heads** have been evaluated
527 using the concept of damage tolerance. Experimental results have shown the suitability of
528 numerical procedures to predict ultimate loads, in general much lower than predicted by
529 considering components without defects.

530 As a summary, two issues should be remarked; one related with design and another with
531 material selection:

- 532 • **Designing by using the concept of damage tolerance can be useful for optimizing the**
533 **design of an element, although the defect characteristics can be difficult to define; in**
534 **the case of the anchor head**, analysed in this work, the relevance of the component depth
535 and, particularly, of the transition angle θ , **has** been proved: smooth transitions provide
536 higher **breaking loads** for the same defect, thus increasing the damage tolerance of the

537 element. This fact is illustrated in Fig. 13 where, for tiny cracks, the stress intensity
538 factor is around two or three times smaller when the transition joint is circular with
539 respect to more abrupt transitions, such as those with θ angle equal to 90°, 130° or 150°.
540 This is related to a higher stress concentration in abrupt changes of thickness, which
541 leads to a reduction of the breaking load, thus remarkably reducing the damage
542 tolerance of the element.

- 543 • Finally, a needed property for damage tolerance design is the material toughness under
544 the appropriate environment, a property not always available, since classical design —
545 ignoring cracks— only needs Young modulus, yield stress, ultimate tensile strength and
546 ductility. As the concept of damage tolerance becomes more usual, these data will be
547 more accessible, meanwhile it is recommended to perform toughness tests with the
548 material of choice [32, 33] and under the scheduled working environment.

549 These suggestions should help in designing safer structural components.

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551

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Declaration of interests

The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

The authors declare the following financial interests/personal relationships which may be considered as potential competing interests: